

# GROUNDWATER FLOW and SEEPAGE

Engineering Department of  
Hydraulic Structures & Environment

Master of Hydraulics  
Academic Year 2012 – 2013

Course n°1: Introduction

Dr. Robert WOUmeni

# FOREWORD

Groundwater flow and Seepage problems are encountered

- in **Civil and Environmental Engineering** (i.e. flow through or under dams, toward wells or drains, around a sheetpile wall or a clay blanket,...);
- in **Hydrology** (i.e. infiltration of rain water in soils, water exchanges between embankment and rivers,...);
- and also in **Hydrogeology** (i.e. long term subsurface flows, ground water resource and quality, polluted soils rehabilitation, pumping,...).

# FOREWORD

In all these problems, we have to deal with a fundamental principle which is **the Darcy's Law**.

The **objective** over an investigated area can be resumed as the following:

- If the soil hydrodynamic properties are known, we can **calculate the groundwater discharge (Q), the head (H) and pore pressure (P)**, and then anticipate the occurrence of a breaking (Civil Engineering) or make the assessment of the water resource (Hydrogeology).
- If the spatial profile of groundwater is known for a given time (i.e. by means of field measurements) then the **soil hydrodynamic properties can be estimated**.

# FOREWORD

So using Darcy's Law, we would like through this course, to  
**develop the following skills:**

- Calculate the groundwater discharge, head and pressure, under **various configurations** (i.e. spatial geometry, anisotropy, saturated conditions, transient flow,...) with **different methods** graphical, analytical and numerical.
- Make the **critical thinking** of a numerical model (i.e. Hydronap, Modflow, Seep-2D);
- Estimate the **hydrodynamic properties** of soils (i.e. hydraulic conductivity  $K$ , storage coefficient  $S$ , porosity  $n$ , transmissivity  $T$ ), from field measurements (SERES Stat.).

# FOREWORD

The level throughout this course will be placed between undergraduate (Darcy's Law, Flow Net calculations,...) and postgraduate (complex potentiel, transient flow,...).

Hard working students could at the end, acquire an interesting background and be ready to join companies involved in the following topics :

- Earth Dams design, construction and inspection (EDF,...)
- Management of Embankment and Rivers (CNR,...)
- Soils and Environmental Studies in relation with groundwater or pollution (Antea, Artelia, Burgeap, Safege,...)
- Research issues (CEA, Cemagref, Andra, Brgm,...)

# FOREWORD

This course will be taught on the following basis :

- $9 \times 2 = 18$ h of main courses (theoretical),
- $4 \times 2 = 08$ h of tutored exercices,
- $1 \times 4 = 04$ h of a field work (practical, on the SERES Station).

The **final grade** will consist of an exam (1.5 ects), and the practical work report (0.5 ects)

The course documents will be available on **a printed form**. Whatever, a French version will be placed on the intranet.

The students can feel free to ask their questions either in French or in English.

**New:** Some courses will end with **a written quiz** for 5 to 10 minutes. It will help, to **capture what the students understand** and where are difficulties.

# SUMMARY

- INTRODUCTION
- HYDRODYNAMIC PROPERTIES (exercices)
- STEADY STATE FLOW (equations, analytical solutions)
- STEADY STATE FLOW (flow nets, exercices)
- STEADY STATE FLOW (complex potentiels, wells group)
- STEADY STATE FLOW (numerical modeling)
- TRANSIENT FLOW (drainage, groundwater pumping,...)
- TRANSIENT FLOW (numerical modeling)
- HYDRAULIC CONDUCTIVITY FIELD MEASUREMENTS



# 1: INTRODUCTION

As mentioned before, **Drainage** and **Seepage** will be a matter of concern in this course.

The 3 mains causes of **dam's breaking** are:

- Structural failure (cracking resulting from excessive strain)
- Overtopping failure (due to heavy flooding conditions)
- Seepage failure (**excessive pore pressure** or **high flow rate**)

Seepage flows are unavoidable in earthen dams. It has to be controlled, if we want to anticipate increasing troubles with sometimes significant damages. This can be achieved with **various equipments** (i.e. toe drains, a clay core, a chimney drain, an upstream impervious blanket, a cutoff wall for pervious foundations, an upstream sloping core, etc...). These equipments help keeping dry the downstream slope of the dam or reducing the flow rate.



# INTRODUCTION



Embankment dams (or earthen dams) are designed to prevent flooding along rivers, or collect water resource for hydropower, drinking water, irrigation and recreation.

Serre Ponçon Dam (photo on the right) is the biggest embankment (remblai) dam in Europe, with the following settings: Height=123,5m, Base width=650m, Crest width=10m, Length=600m, Zoned earth dam equipped with a clay core, Material Volume=14 millions m<sup>3</sup>, Storage=1,27 billion m<sup>3</sup>.

# INTRODUCTION



The Pomme de Terre Dam: an earth and rockfill embankment; 7,240 ft long, 155 ft in height; covering 7,820 acres in the Missouri River Basin (Kansas State). Equipped with a compacted clay core. Construction 1957-1961 for \$15 millions. Fonction: flood protection and recreation.



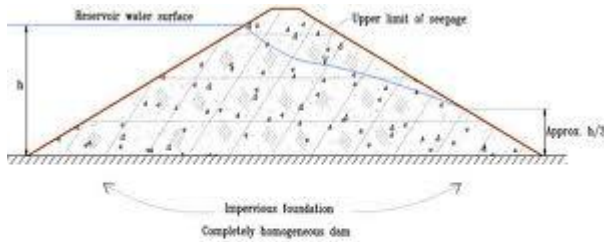
The Banasura Sagar Dam: the biggest earth dam in India, 2<sup>nd</sup> in Asia; 3300 ft long.



Benmore Dam (New Zealand): the largest earth dam in south hemisphere, built for hydroelectricity.

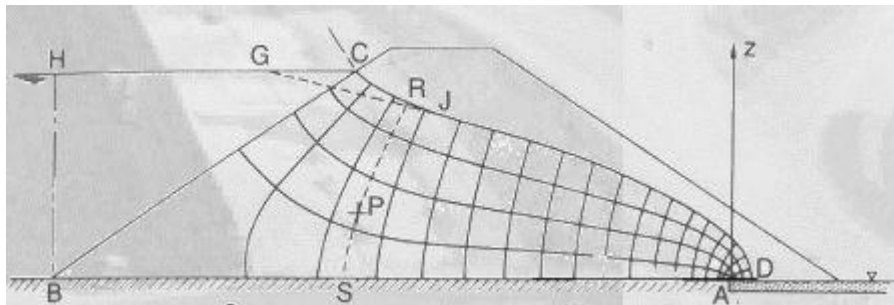


# INTRODUCTION

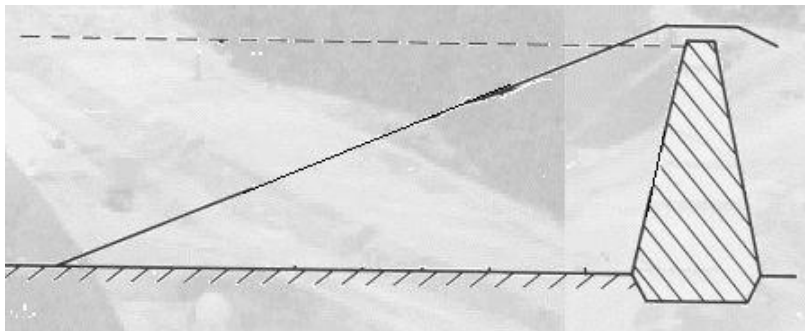


Seepage flow through earth dam with no filter at the dam toe

Seepage through a dam, without a drainage system. The phreatic line (or upper saturation line) is shown. The foundation is considered as impervious.



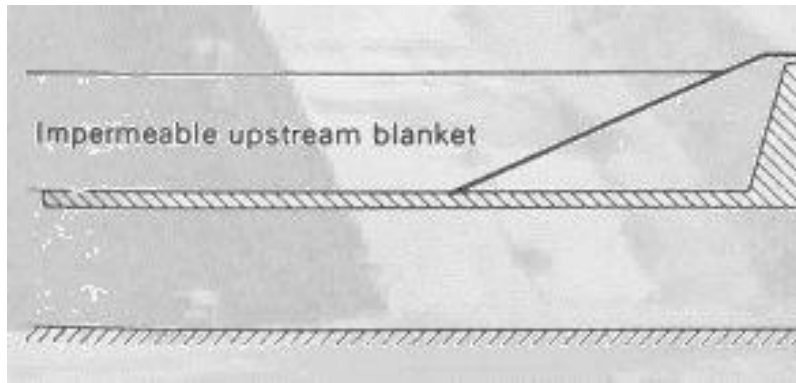
A dam equipped with a toe drain. The flow network (graphical solution) is shown. The foundation is considered as impervious.



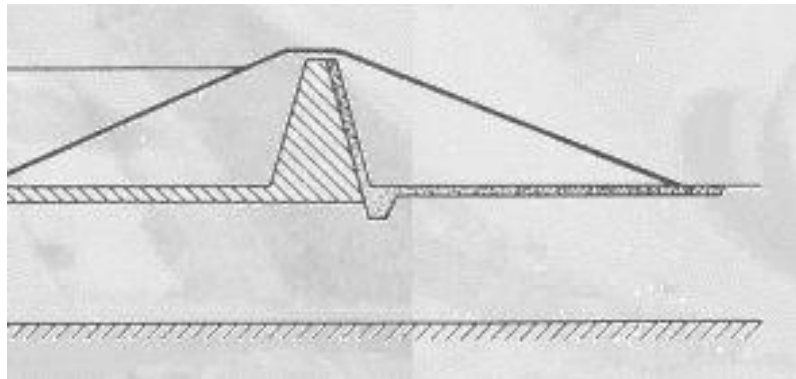
A zoned dam equipped with a central clay core. The foundation is considered as impervious.



# INTRODUCTION



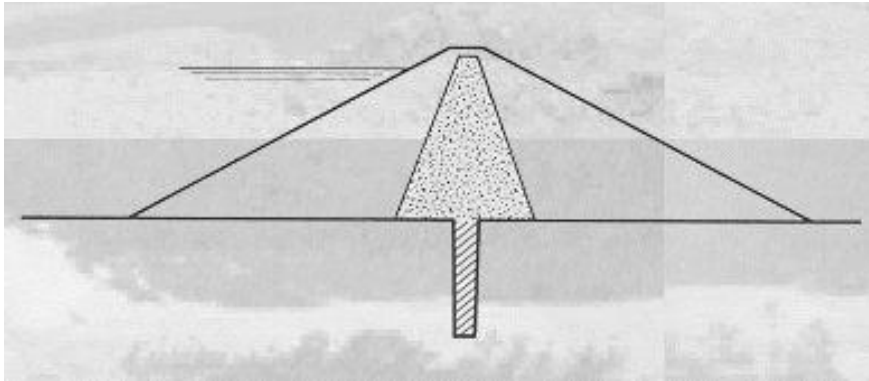
This dam is equipped with a clay core and an **upstream impervious blanket**.



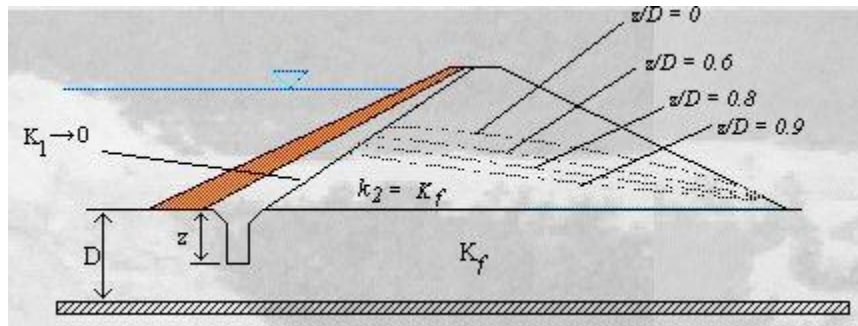
Here we have a clay core and a **chimney drain** completed with a **downstream pervious blanket**.



# INTRODUCTION



A zoned dam equipped with a central clay core and a **cutoff wall**, in the case of pervious foundation.



A zoned dam equipped with a **sloping clay core** set upstream, and a partial cutoff wall, in the case of pervious foundation. The different phreatic lines with respect to wall size are shown.



# 1: INTRODUCTION

In seepage failures, we include **Slide** and **Piping** failures.

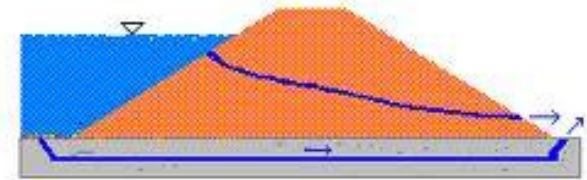
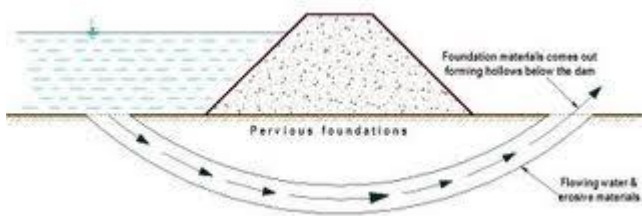
Seepage can cause slide or slope failure by creating high pressures in the soil pores or by saturating the slope. The shear resistance is reduced and the soil become weak. This can happen after long or heavy rainfall on the downstream slope, or rapid drawdown near upstream slope.





# 1: INTRODUCTION

If the seepage velocity is uncontrolled, it can progressively erode soil from the embankment or its foundation, resulting in rapid failure of the dam. **Erosion of the soil begins at the downstream** side of the embankment, either in the dam proper or the foundation, progressively works toward the reservoir, and eventually develops a direct connection to the reservoir. This phenomenon is known as “**Piping** (Renardage).”



This dam is hazardous.



# 1: INTRODUCTION



Buget Dam  
(France). A piping  
from downstream.

Courtesy : D. LAUTRIN



Gourdon Dam (France).  
A piping from upstream.

Courtesy : D. LAUTRIN



St-Julien Dam  
(France). A piping  
along outlet works.

Courtesy : P. MERIAUX, P. ROYET

Piping action can be **recognized** by an **increased seepage** flow rate, the discharge of **muddy water**, **sinkholes** on the embankment, a **sand boil** (Boulance) downstream, or sometimes a **whirlpool** in the reservoir (when it's too late to intervene).





# 1: INTRODUCTION



Teton Dam (1976, Idaho, USA).



Saint-Aignan Dam (France).



Cowlitz Dam (USA).



Acidic seepage failure near Donana's National Park (1998, Southern Spain).



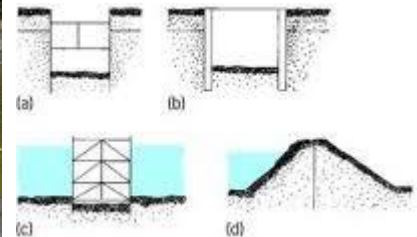
# 1: INTRODUCTION

Some other examples of **groundwater flows** in Civil Engineering are given by **Excavation dewatering** (pumping works):



Many wellpoints are set around the working area and their individual drawdown are superimposed for the total dewatering.

Flow under a **sheetpile** (palplanche), Flow under a **cofferdam** (batardeau).

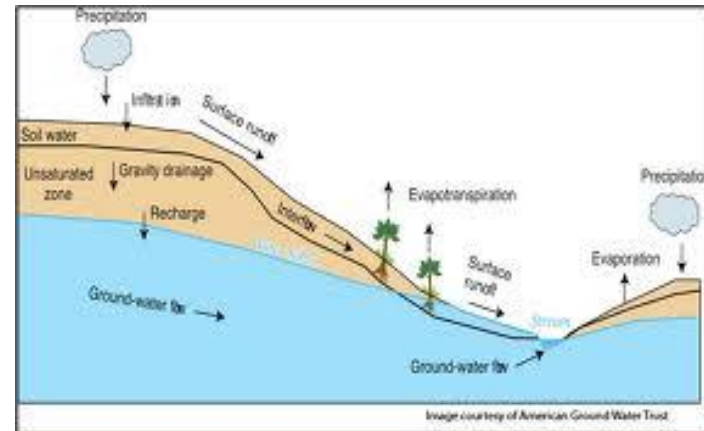




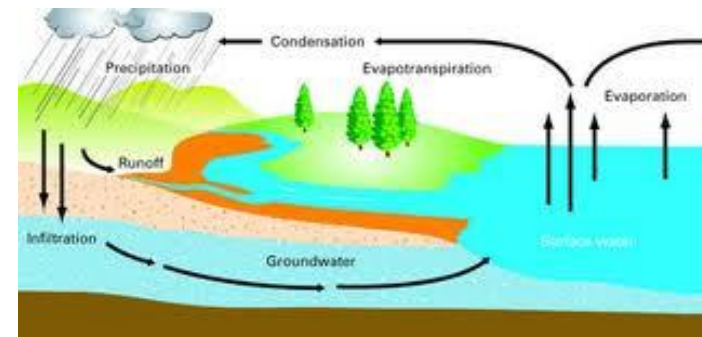
# 1: INTRODUCTION

After Seepage in Civil Engineering, we will next focus on Drainage in Groundwater Resource and Environmental Issues.

Groundwater flows through pervious aquifer, from recharge zones upstream where infiltration takes place, to discharge zones downstream which can be linked to a surface body water (i.e. river, lake or even ocean).



This is part of the global water cycle, which include, others hydrological flux terms such as evaporation, precipitation and run off.

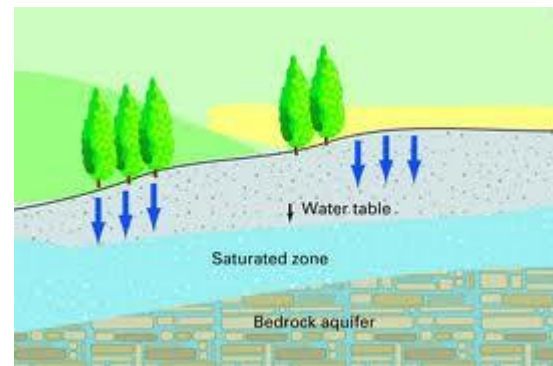
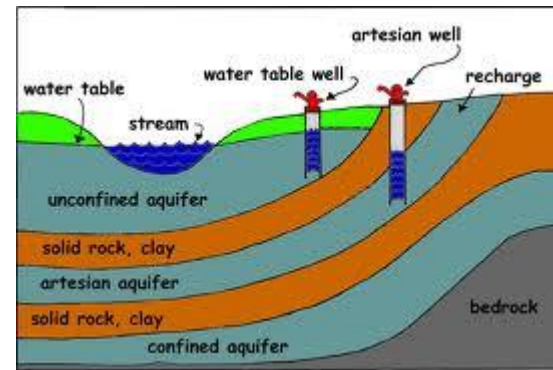


# 1: INTRODUCTION

The flow rate and water quality depend on the aquifer type :  
confined, unconfined or fractured.

In unconfined aquifers, the upper saturated line is at atmospheric pressure (then set to zero) and exchanges with stream water are frequent. Whereas for confined aquifer, groundwater is kept tight between 2 impervious layer (solid rocks or clay).

The phreatic line is also called water table. One may notice that very often, the upper part of a solid bedrock presents some cracks, then leading to another type of aquifer with higher flow rate.



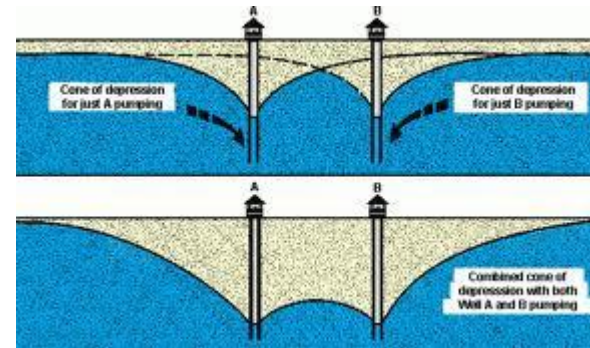
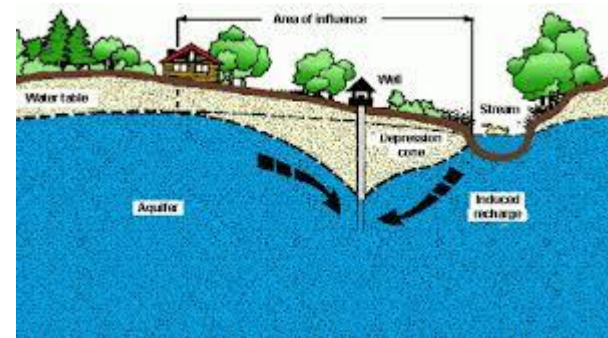


# 1: INTRODUCTION

A key engineering operation with groundwater management is undoubtedly pumping.

With a given pumping flow rate, what drawdown in space and time, will we have ? Does it make a difference to pump near a river ? What is the influence of water table slope, type of aquifer,...? We will make efforts to address these questions.

How to calculate spatial drawdown in the case of a group of pumping ? We will also see the theory of imaging wells.



# 1: INTRODUCTION

It's worthwhile to make a difference between a well and a borehole

A well is often made in masonry, not too deep (a few ten meters), and with a diameter of about 1 meter or even more.



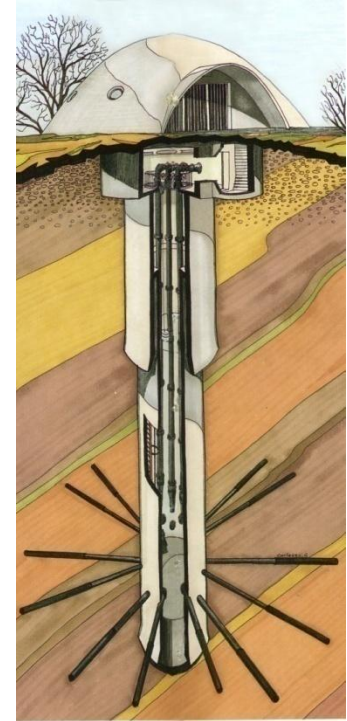
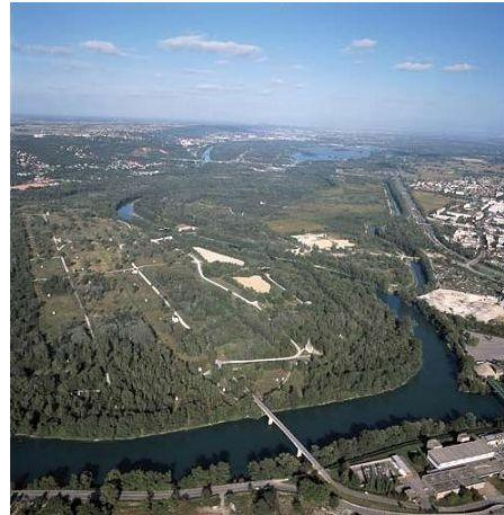
A borehole consists of steel material, can be drilled up to hundred or thousand of meters deep, with quite small diameter (i.e. 0.60 m)



# 1: INTRODUCTION

A **wellfield**, is a protected area where many wells are operated to supply a city with drinking water.

At Crepieux-Charmy (France) not less than 114 wells are pumped everyday to provide about 930000 people with 275000 m<sup>3</sup> (61% of the supply capacity). Many recharge zones are present on the site to anticipate a chemical pollution in the nearby stream. This is one of the largest wellfield in Europe (375 ha).



4 wells with **radiant drains** at the Rochefort wellfield (Grenoble), can extract from 2000 to 3000 m<sup>3</sup>/h at 30m depth, in the alluviums of the Drac River.





# 1: INTRODUCTION

Our **teaching tools** will include: The *SERES Station* and 2 numerical codes *HYDRONAP* + *MODFLOW* based on *finite differences scheme of an iterative resolution*.



The SERES Station



# 1: INTRODUCTION

## SERES Station History :

SERES Station has been designed and built up by Dr. Robert WOUMENI, as a platform for experimental field testings on seepage and drainage.

- 1994, **Pumping tests** in an unconfined aquifer for field hydraulic conductivity measurements were ready.
- 1995, Stepwise **Well Tests** for the assessment of well performance have been worked out.
- 2000, **Tracer tests** for groundwater pollution were achieved for dispersivity measurements in the field.
- 2002, Some **hydraulic rehabilitation testings** by groundwater removals were done.
- 2008, We created a **Pilot Scale Aquifer Model**, for toxic contaminant transport tests.
- 2010, A study for **geothermal application** on the site was launched.
- 2011, Preliminary results on our **Vertical Electrical Sounding Tests** (geophysical method) for pollutant studies were obtained;
- 2014, We will design and carry out some **Infiltration Tests** for the studies on wastewater filtration systems.



# 1: INTRODUCTION

## SERES Station Location :

The Seres Station is located on the Grenoble University Campus, at about 300m from the Isere River.

An unconfined sandy aquifer (not used for drinking water) is underneath at about 3m depth.

Latitude: 45°11' 47.26" N

Longitude: 5° 46' 11.86" E

Elevation: 210 m

A surface area near than 1 ha is available for testings on the groundwater resource.



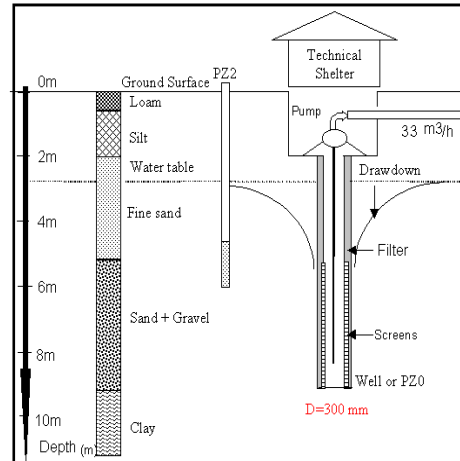


# 1: INTRODUCTION

## SERES Station Equipments :



A view of the borehole.



Lithology



The discharge pipe.



An observation Well



Our Pilot Scale Aquifer Model



A 33m<sup>3</sup>/h Pump.



# 1: INTRODUCTION

## SERES Station Devices :



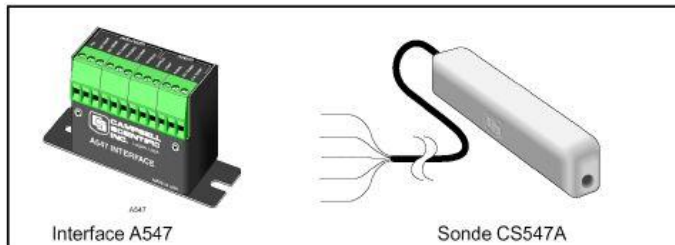
A Data logger



Pressure transducer probe



Water level device



Conductivity probe



Spectrophotometer



Electrical Sounding device 28





# 1: INTRODUCTION

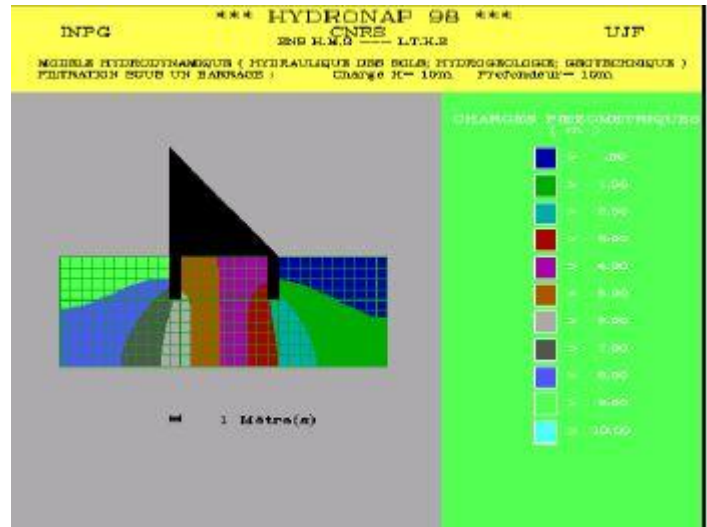
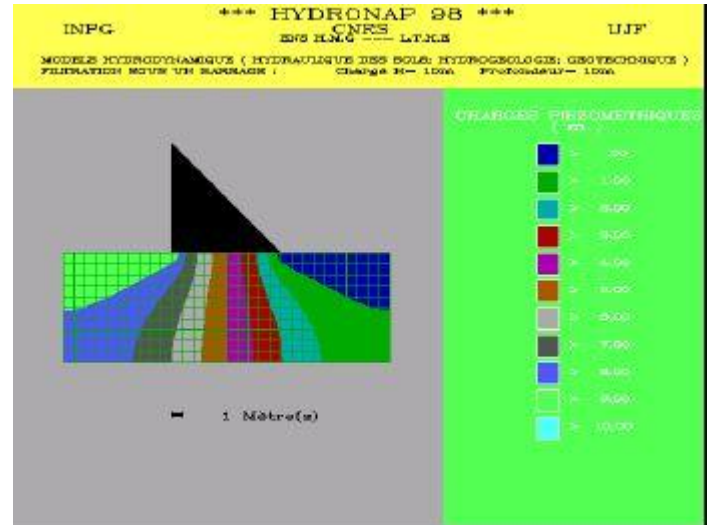
Numerical code *HYDRONAP* has been designed and built up in 1998 by Dr. R WOUmeni.

After taking 2h to learn how to use the code, the students can improve their understandings in seepage and drainage problems.

The following items can be addressed

- Seepage in dams foundations;
- Influence of cutoff walls;
- Transient Pumping;
- Pumping near a river or wall;
- Designing a hydraulic barrier;
- Flow under cofferdams, etc..

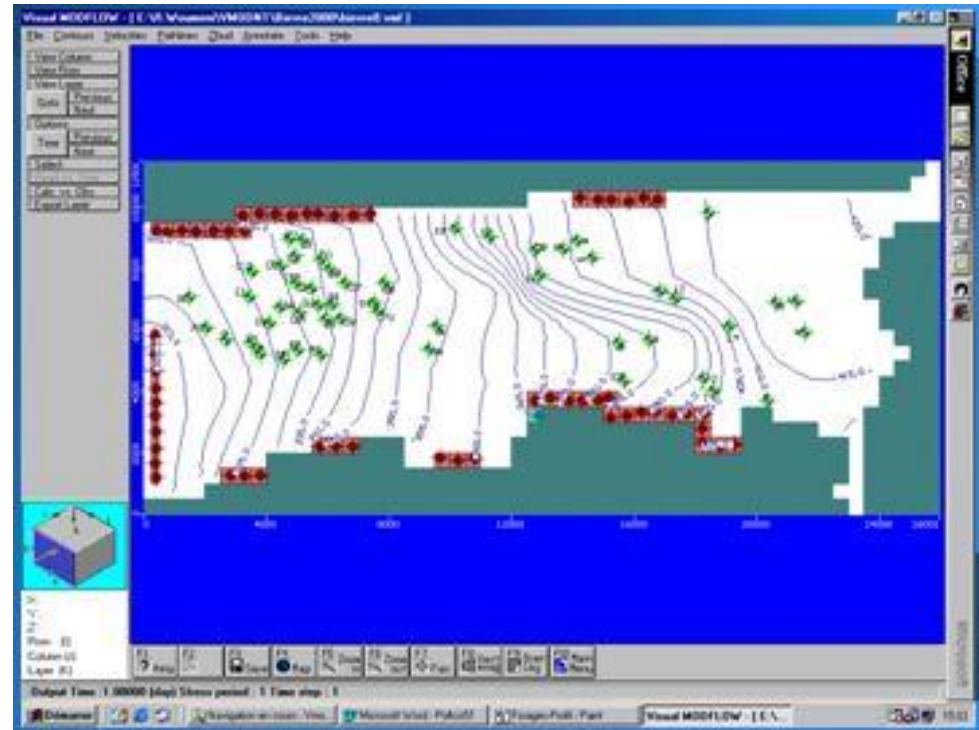
Robert WOUmeni



# 1: INTRODUCTION

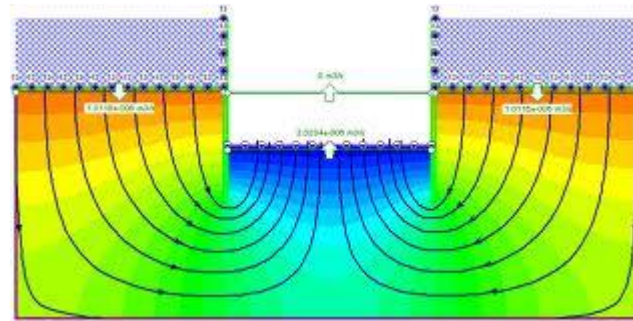
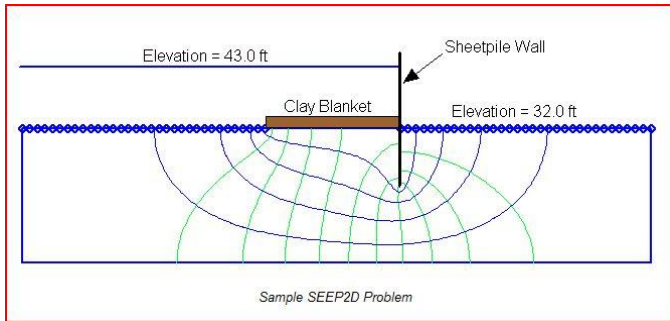
The numerical code **MODFLOW** is certainly the most popular numerical code used for groundwater management studies.

On the example hereby, groundwater levels for the Bievre plain, range from 400m (right) to 300m (left side). The wells around (in red colour) help to simulate inflow or outflow on the boundaries.



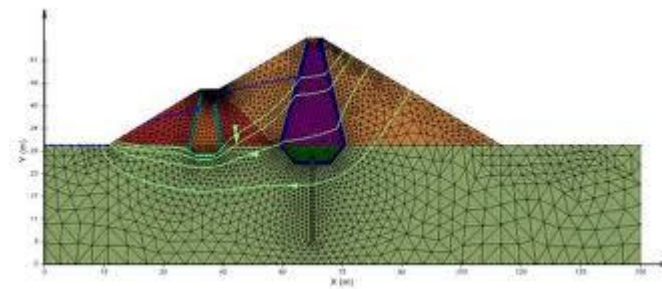
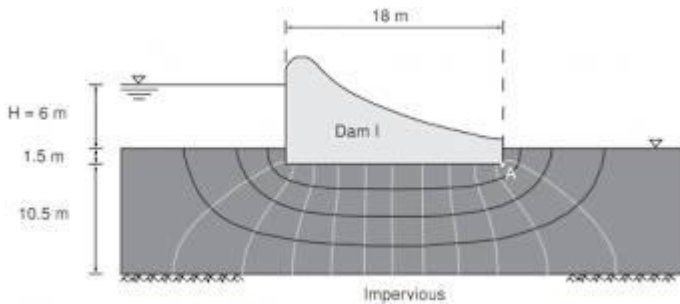
# 1: INTRODUCTION

Let us mention other numerical codes which can be useful in this course : Seep2D, Plaxis, SVFlux, Phase2, Feflow,.....



Flow around a sheetpile with **Seep2D** (by the Seepage Analysis Group, US).

Seepage flow toward a cofferdam with **Phase2** (by RocScience, Canada)



Flow under a concrete dam with **Plaxis 2D** (by Plaxis BV, Netherland)

Flow through a clay core earth dam with **SVFlux** (by SoilVision, Canada)



# 1: INTRODUCTION

Then Seepage and Drainage cover a large range of applications, and many classical knowlegdes are involved : Geotechnics, Hydrogeology, Hydrodynamics in Porous Media, Soil Mechanics, Fluid Mechanics, Hydrology.

This is very wealthy but also a cause of difficulties, as for a given problem, we need to combined theoretical and practical aspects on a balanced way. The resulting solution may often depend on a variety of factors: Is the flow confined or unconfined ? Is the soil homogeneous ? Do we have a 1D-2D or 3D problem ? Is there some symmetry ? What are the types of boundary conditions ? Are we under steady state or transient conditions ? Is the soil fully saturated ? What type of spatial coordinates would be suitable ?

